

water & sanitation

Department: Water and Sanitation REPUBLIC OF SOUTH AFRICA



Feasibility-level Cost and Implication Analysis Sub-Report

Support of the Water Reconciliation Strategy for the Algoa Water Supply System

March 2020 Revision: 1 Reference: 112546



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DEPARTMENT OF WATER AND SANITATION

Directorates: National Water Resource Planning and Options Analysis

Support of the Water Reconciliation Strategy for the Algoa Water Supply System

Feasibility-Level Cost and Implication Analysis Final: March 2020

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SUPPORT OF THE WATER RECONCILIATION STRATEGY FOR THE ALGOA WATER SUPPLY SYSTEM

APPROVAL

Title:Feasibility-Level Cost and Implication AnalysisConsultant:Aurecon South Africa (Pty) LtdReport status:FinalDate:March 2020

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Feasibility-level Cost and Implementation Analysis Sub-Report Project 112546 March 2020 Revision 1Page iii



water & sanitation

Department: Water and Sanitation REPUBLIC OF SOUTH AFRICA



Feasibility-level Cost and Implication Analysis Sub-Report

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DEPARTMENT OF WATER AND SANITATION

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Department of Water and Sanitation Directorates: National Water Resource Planning and Options Analysis

SUPPORT OF THE WATER RECONCILIATION STRATEGY FOR THE ALGOA WATER SUPPLY SYSTEM

APPROVAL

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Consultant	:	Aurecon South Africa (Pty) Ltd
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Document Index

Reports that will be produced as part of this Study are indicated below.

Bold type indicates this Report.

Report Index	Report Number	Report Title
1		Inception
2		Water Requirements Chapter
3		Identifications of Options for Balancing Storage Chapter
4		Environmental Constraints Analysis
5	P WMA 15/N40/00/2517/1	Topographical Survey
7a	P WMA 07/N40/00/2619/2	Lower Coerney Dam Geotechnical Survey
7b	P WMA 07/N40/00/2619/1	Upper Scheepersvlakte Dam Geotechnical Survey
7c	P WMA 07/N40/00/2619/3	Lower Coerney Dam Supplementary Geotechnical Survey
8		Desktop assessment of short-listed options Chapter
9	P WMA 15/N40/00/2517/3	Options Analysis Report
10		Layout and Affected Land and Infrastructure
11		Feasibility-level engineering design - balancing dam
12		Feasibility-level engineering design - conveyance infrastructure
13		Feasibility-level cost and implementation analysis
14		Implementation Support
15	P WMA 15/N40/00/2517/4	Feasibility Study Report
16	P WMA 15/N40/00/2517/5	Stakeholder Participation: Feasibility Study

Executive Summary

Introduction

The objective of the Feasibility Component of the Support of the Water Reconciliation Strategy for the Algoa Water Supply System study is to:

- limit risks of shortfall in supply to the Nelson Mandela Bay Municipality (NMBM) and the Lower Sundays River Government Water Scheme (LSRGWS),
- remove potential operating system constraints for the sustainable delivery of bulk Orange River water supply to the LSRGWS and NMBM, for water requirements up to 2040, and
- limit operational risks to acceptable levels.

The existing Scheepersvlakte Balancing Dam is a balancing facility for water supply to the LSRWUA and the NMBM, and for emergency supply.

The focus of the investigation is on providing additional balancing storage in addition to the existing Scheepersvlakte Balancing Dam, which has lost storage due to siltation.

The main purpose of the proposed new balancing dam, at the Coerney site, is to eliminate the operational and balancing storage limitations imposed by Scheepersvlakte Dam.



Balancing dam feasibility level cost estimate

The costing and main design assumptions are discussed, concluding in the cost estimate for the construction of the balancing dam as well as the associated costs (access, electricity, etc.) and lastly the professional fees, as shown in **Table E1**.

Amount, rounded No Description (million) 1 Construction cost R 156.82 2 Preliminary and General Items (50%) R 78.41 Sub total 235.24 R 3 R Access and electrical supply 2.75 Sub total R 237.99 4 R Contingencies (25%) 59.50 Total R 297.48

Table E1: Balancing Dam cost (excluding 15% VAT)

The operation and maintenance costs are estimated as a percentage of the construction cost, divided into three categories: civil, mechanical and the dam. The cost of the river diversion, preliminary and general cost items and professional fees are excluded from the construction values below. The estimated annual operation and maintenance costs are as shown in **Table E2**.

Table E2: Dam	Operation and	Maintenance costs	(excludina	15% VAT)
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No	Description	Percentage	Construction Cost (million)		Annual Cost (million)	
1	Civil works	0.5%	R	89.653	R	0.448
2	Mechanical works	4.0%	R	18.750	R	0.750
3	Dam (embankment)	0.25%	R	78.564	R	0.196
	Total		R	186.967	R	1.394



Conveyance infrastructure cost estimate

The costing and main design assumptions are discussed, concluding in the cost estimate for the construction of the conveyance infrastructure, as shown in **Table E3**.

No	Description	Amount, rounded (million)	
1	Kirkwood Canal off-take	R	1.87
2	Inlet/outlet to Coerney Dam	R	6.83
3	Main connecting pipeline	R	32.76
4	Tie-in to Nooitgedagt pipeline	R	1.28
5	Crossing of Middle Addo canal	R	0.81
6	Syphon under Sundays River	R	11.21
	Sub total (a)	R	54.76
7	Preliminary and General Items (50%)	R	27.38
	Sub total (b)	R	82.14
8	Contingencies (15%)	R	12.32
	Total	R	94.46

Table E3: Conveyance infrastructure cost (excluding 15% VAT)

The operation and maintenance costs are estimated as a percentage of the construction cost, divided into two categories: civil and mechanical. The cost of contingencies is included, and the costs of the professional fees and preliminary and general items are excluded from the values below. The operation and maintenance costs for the conveyance infrastructure are shown in **Table E4.**

Table E4: Conveyance Infrastructure Operation and Maintenance Costs (excluding15% VAT)

No	Description	Percentage	Construction Cost (Rand million)		Annual Cost (Rand million)	
1	Civil works	0.5%	R	56.809	R	0.284
2	Mechanical works	4.0%	R	6.164	R	0.247
Total		R	62.972	R	0.531	



Other miscellaneous project costs

The other project costs include land acquisition for the dam and pipeline, professional fees, which includes design, applications and licencing, etc. as shown in **Table E5**.

No	Description	/ (Ra	Amount nd million)
1	Land acquisition	R	21.950
2	Professional fees for dam (10% of construction cost)	R	29.748
3	Professional fees for conveyance infrastructure (10% of construction cost)	R	9.446
	Total	R	61.144

Table E5: Other miscellaneous costs (excluding 15% VAT)

Total project cost

Total project costs are shown in Table E6.

Table E6: Total project costs

No	Description	Amount		
1	Balancing dam	R	297 481 000	
2	Conveyance infrastructure	R	94 456 000	
3	Professional fees	R	39 194 000	
	Value Added Tax (15%)	R	64 670 000	
4	Land acquisition	R	21 950 000	
	TOTAL (January 2020 prices)	R	517 753 000	

The project cost estimate for commencement in 2025, with escalation of 6.5% per annum from the base year of 2020 is shown in **Table E7**.



Table E7: Total project costs (including 15% VAT)

Contents

1	Introduction and background	1
1.1	Study Objective	1
1.2	Purpose of this Sub-report	1
1.3	Background	1
1.4	Content of this Report	2
2	Coerney Dam cost estimate	3
2.1	Embankment materials	3
2.2	Foundation treatment	4
2.3	Spillway	4
2.4	Inlet/Outlet works configuration	4
2.5	Outlet tower	5
2.6	Electrical supply requirements	5
2.7	Access	5
2.8	Instrumentation	5
2.9	River diversion	5
2.10	Construction cost estimate	5
2.11	Operation and maintenance costs	6
3	Conveyance infrastructure cost estimate	8
3.1	New offtake at Kirkwood primary canal	8
3.2	Inlet/outlet to proposed Coerney Dam	8
3.3	New connecting pipework	9
3.4	Tie in to existing Nooitgedagt supply pipeline	9
3.5	Crossing of the Middle Addo Canal	9
3.6	Syphon under Sundays River	9
3.7	Construction cost estimate	10
3.8	Operation and maintenance costs	10
4	Other miscellaneous project costs	12
4.1	Land acquisition	12
4.2	Professional fees	12
4.3	Cost estimate	12
5	Project cost estimate	13

Appendices

Appendix A: Cost estimate for the balancing dam Appendix B: Cost estimate for the conveyance infrastructure

Tables

Table 2-1: Embankment material quantities and sources	3
Table 2-2: Cost estimate summary for the balancing dam (excluding VAT)	6
Table 2-3: Operation and maintenance costs for the balancing dam (excluding VAT)	7
Table 3-1: Cost estimate summary for the conveyance infrastructure (excluding VAT)	10
Table 3-2: Operation and maintenance costs for the conveyance infrastructure (excluding VAT)	11
Table 4-1: Other miscellaneous project costs (excluding VAT)	12
Table 5-1: Project cost estimate	13
Table 5-2: Project cost estimate with escalation	13

Abbreviations

DN	Nominal diameter
DWS	Department of Water and Sanitation
LSRGWS	Lower Sundays River Government Water Scheme
LSRWUA	Lower Sundays River Water User Association
masl	Metres above mean sea level
NMBM	Nelson Mandela Bay Municipality
NOC	Non-overspill crest
RDF	Recommended Design Flood
SEF	Safety Evaluation Flood
WTW	Water Treatment Works

1 Introduction and background

1.1 Study Objective

The objective of the Feasibility Component of the Support of the Water Reconciliation Strategy for the Algoa Water Supply System study is to:

- limit risks of shortfall in supply to the Nelson Mandela Bay Municipality (NMBM) and the Lower Sundays River Government Water Scheme (LSRGWS),
- remove potential operating system constraints for the sustainable delivery of bulk Orange River water supply to the LSRGWS and NMBM, for water requirements up to 2040, and
- limit operational risks to acceptable levels.

The focus of the investigation is on providing adequate balancing storage for supply to the NMBM, to limit risks of shortfall in supply.

1.2 Purpose of this Sub-report

The purpose of this sub-report is to describe the costing parameters, as well as the assumptions applicable to the feasibility design of the balancing dam and conveyance infrastructure in order to determine their respective construction, and operation and maintenance costs.

The content of this sub-report will form a Chapter/s of the Feasibility Design Report.

1.3 Background

Following the expected completion of the Nooitgedagt Water Treatment Works (WTW) Phase 3 in 2022, the WTW will have a maximum capacity of 210 Ml/day. The scheme has been designed to cater for peak/back-up supplies from the Nooitgedagt WTW at times when the older infrastructure, from sources to the west of Port Elizabeth, will be requiring maintenance or emergency repairs.

After investigation of a number of potential dam sites, as documented in the Options Analysis Report (DWS, 2019), the Lower Coerney Dam site was found to be the most favourable site

for the proposed new balancing dam and has been recommended for feasibility design. The proposed dam is referred to as 'Coerney Dam' in the sub-report and future reports, as there is no Upper Coerney Dam.

1.4 Content of this Report

The report is divided into three main parts, discussing the cost estimate of the:

- Coerney Dam,
- Conveyance infrastructure, and
- Other costs, which include legislative processes, professional fees and land acquisition.

Finally, the report concludes with a Project cost estimate, which combines the preceding cost estimates of the various components. The various chapters in this report and their content are briefly described hereunder.

Chapter 1: Introduction and background

Provides a brief introduction and background to the project and report.

Chapter 2: Coerney Dam cost estimate

Describes the main assumptions and parameters used in estimating a cost for the construction of the dam, as well as an estimate of the operation and maintenance costs.

Chapter 3: Conveyance infrastructure cost estimate

Describes the main assumptions and parameters used in estimating a cost for the construction of the conveyance infrastructure, including an estimate of the operation and maintenance costs.

Chapter 4: Other miscellaneous project costs

Describes the main assumptions and parameters used in estimating a cost for the other anticipated project costs, as well as an estimate of the operation and maintenance costs relating to the infrastructure aspects of the items covered in this section.

Chapter 5: Project cost estimate

Provides a total project implementation cost estimate for the dam, conveyance infrastructure and other associated project costs.

2 Coerney Dam cost estimate

The dimensions, assumptions and other factors affecting the costing of the various components of the balancing dam are discussed below.

2.1 Embankment materials

The proposed embankment dam design has typical embankment slopes of 1V:3H upstream and 1V:2H downstream. The dam has a crest length of 600 m and a crest width of 5 m. The lowest level at the valley bottom is 81.5 masl with a non-overspill crest (NOC) level of 102.0 masl, which results in a maximum wall height of 20.5 m.

As part of the site preparations, the topmost 0.3 m - 0.5 m of soil will need to be removed from the embankment footprint (4.3 ha) before placement of the embankment fill material. This topsoil should be stockpiled for use on disturbed areas.

The total embankment volumes are summarised in **Table 2-1**. The material for the embankment homogeneous fill zone will be excavated from within the dam basin, which is within normal free-haul distance of 1 km, and no allowance is thus made for overhaul. It has been assumed that the essential excavations from the spillway and discharge channel (approximately 94 000 m³) will be used in the embankment construction, with no double handling of these materials.

Zone	Volume	Source
Homogeneous fill	350 000	Dam basin
Upstream rip-rap	12 650	Imported
Sand and gravel filters	5 100	Imported
Rock toe drain	2 370	Imported

Table 2-1:	Embankment	material	quantities and sources
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Materials, such as fine and coarse aggregate for concrete and sand for filters, were not found in the basin and will have to be imported from commercial sources. No project-specific borrow areas were identified or investigated. The current costing makes allowance for their importation from commercial sources.

A few possible commercial sources for sand and coarse aggregates have been identified, but all are located some distances away from the Coerney Dam site. These are discussed in the *Feasibility-level Design: Balancing Dam Sub-Report* (DWS, 2020).

2.2 Foundation treatment

The 'groutability' of the weathered rocks encountered for the foundations of the dam core trench, outlet pipe encasement and outlet tower is uncertain.

No allowance has been made for consolidation or curtain grouting under the embankment. Some allowance is made for curtain grouting along the spillway overflow structure centreline.

Special mention should be made of the mudrock, which is susceptible to slaking. This rock will require immediate protection after exposure to prevent deterioration before construction/covering will commence. No allowance has been made for this.

2.3 Spillway

The spillway is excavated into the left abutment. The spillway overflow sill is an ogee-shaped mass gravity concrete overflow weir that is 6.35 m high and 50 m long. In general, the upper soils are underlain by weak bedrock that would be susceptible to erosion. It is thus required that the spillway side channel and discharge channel are lined with reinforced concrete. The side channel liner extends to the NOC, as it will also act as a retaining wall where the channel abuts to the embankment. The thickness of the liner is 0.4 m and the discharge channel is lined to the safety evaluation flood (SEF) flow depth of 1.7 m.

The discharge channel ends in a rectangular stilling basin. The basin is 20 m long with 6 m high side walls, which have a slight batter (1H:5V).

2.4 Inlet/Outlet works configuration

The outlet works of the dam will be located on the left abutment (eastern bank) of the valley. The pipe for supplying water to and from the dam will bifurcate into an inlet and outlet branch at the downstream outlet chamber, which is situated at the downstream toe of the embankment. The costing for this pipe, up to the start of the encasement, is described in Section 3, which addresses the conveyance infrastructure. The inlet/outlet pipes through the embankment have been included in the dam costing. The two stainless steel DN 1 m inlet/outlet pipes pass through the embankment, where they are encased in reinforced concrete.

2.5 Outlet tower

The inlet and outlet pipes will be connected to a circular (4 m internal diameter) reinforced concrete wet-well outlet tower. The tower is founded on a concrete base slab 1 m thick, extending 1.5 m past the wall perimeter. The walls are an average of 0.6 m thick. The tower is provided with two intake levels, which can be controlled with gates operated from the top of the tower. The tower will be accessed via a steel pedestrian space frame bridge with a mid-length column dividing the bridge into two spans.

The access bridge, intake gates, gantry and other mechanical equipment in the tower are accounted for with lump sum values.

2.6 Electrical supply requirements

There is currently no design for the electrical supply to the proposed dam location. It is presumed that the supply to the Scheepersvlakte Dam can be extended to the Coerney Dam site. A lump sum allowance has been made in the costing for this extension.

2.7 Access

There are some gravel tracks on either side of the valley of the proposed dam site. It is proposed that the track on the right abutment leading from Scheepersvlakte Dam be upgraded. An overall cost for access roads has been made on a per-kilometre rate and provision of 1.5 km of roads.

2.8 Instrumentation

A number of simple monitoring instruments were discussed and proposed in the feasibility design of the dam. A percentage allowance in the costing has been included to allow for these items.

2.9 River diversion

The river diversion strategy for the construction of Coerney Dam is relatively rudimentary due to the apparent absence of regular flow in the river channel. A lump sum provision has been made in the cost estimate to account for the provisions and risks associated with the river diversion to deal with water and floods. The estimate is a lump sum based on the size of the diversion embankment required and an all-in rate.

2.10 Construction cost estimate

A detailed cost estimate for the balancing dam is provided in Appendix A.

Table 2-2 shows a summary of the costs for the various components and types of work discussed above.

No	Description	Amount (to nearest 1000)	
1	Clearing	R	195 000
2	River diversion	R	10 000 000
3	Excavations	R	20 262 000
4	Drilling and grouting	R	677 000
5	Embankment fill materials	R	41 717 000
6	Concrete works	R	52 570 000
7	Mechanical Items	R	22 525 000
8	Miscellaneous	R	8 877 000
	Sub total	R	156 823 000
9	Preliminary and General Items (50%)	R	78 412 000
10	Access and electrical supply	R	2 750 000
11	Contingencies (25%)	R	59 496 000
	Total	R	297 481 000

Table 2-2: Cost estimate summary for the balancing dam (excluding VAT)

2.11 Operation and maintenance costs

The operation and maintenance costs of the various dam components have been included as an annual cost, based on a percentage of the construction value.

The components are divided into three sections.

- Civil works, which includes the concrete spillway, intake tower and access bridge, the access roads and pipelines.
- The mechanical works, which includes hoisting equipment, valves and gates.
- The dam, which includes the embankment and dam basin.

A simple annual cost estimate for the three categories has been determined, based on a percentage of the construction value. The construction value used includes 25% contingencies and excludes the cost of the river diversion and the 50% preliminary and general charge items. The operation and maintenance costs for the dam, based on the above, are shown in

Table 2-3.



No	Description	Percentage	Cons (round	truction value ded to nearest 1000)	Ann (round	ual Amount led to nearest 1000)
1	Civil works	0.5%	R	89 653 000	R	448 000
2	Mechanical works	4.0%	R	18 750 000	R	750 000
3	Dam (embankment)	0.25%	R	78 564 000	R	196 000
	Total		R	186 967 000	R	1 394 000

Table 2-3: Operation and maintenance costs for the balancing dam (excluding VAT)

3 Conveyance infrastructure cost estimate

3.1 New offtake at Kirkwood primary canal

The new offtake from the Kirkwood primary canal comprises a 1.5 m wide adjustable weir, which will allow for regulating of the flow that can be discharged from the canal to the WTW or to the Coerney Dam. The weir level will be adjusted by means of a manually operated sluice gate, which will allow water to discharge from the canal into a wet well. Water will be piped from the wet well through a magnetic flow meter. The display from the flow meter will be positioned next to the adjustable sluice gate, which will allow the weir to be adjusted to discharge a certain flow.

3.2 Inlet/outlet to proposed Coerney Dam

The proposed dam will be supplied from the Kirkwood primary canal through a DN 1400 pipeline, which will also be used to transfer water to the tie-in point on the existing Nooitgedagt WTW pipeline.

The pipe for supplying water to and from the dam will bifurcate into an inlet and outlet branch at the outlet chamber, which is situated at the downstream toe of the embankment. The inlet branch will have an isolation valve for shutting off supply when the dam is full. The outlet branch will be fitted with a non-return valve and an isolation valve, both upstream and downstream. The non-return valve will ensure that water can be 'automatically' supplied from the dam in the event where the inlet has been shut to avoid spilling of the dam when it is full. The isolation valves will ensure that the non-return valve can be serviced while the inlet pipe remains in operation.

The inlet and outlet pipe branches will reduce from DN 1400 to DN 1200 at the bifurcation and reduce from DN 1200 to DN 1000 after the cross connection, which is before passing through the embankment in a concrete encasement.

3.3 New connecting pipework

A DN 1400 steel pipeline is required between the Kirkwood primary canal offtake and the proposed Coerney Dam, as well as from the proposed dam to a tie-in point on the existing DN 1400 pipeline supplying the Nooitgedagt WTW.

The total length of the two steel pipelines is approximately 2 950 m. Based on the limited geotechnical information available, and assuming that the pipes would be manufactured from Grade X52 steel, a 10 mm wall thickness would be required.

3.4 Tie in to existing Nooitgedagt supply pipeline

A connection is required to the existing 1400 mm diameter Nooitgedagt WTW supply pipeline. This connection needs to be located downstream of the existing cross connection on the Scheepersvlakte pipeline as well as downstream of the high point in the existing supply line.

The tie-in will comprise a 1400 mm x 1400 mm equal tee that will be cut into the existing pipeline. The branch of the tee will be fitted with an isolation valve to close the Coerney Dam's supply should maintenance be required on this pipeline.

3.5 **Crossing of the Middle Addo Canal**

The proposed pipeline will need to cross the Middle Addo canal twice. It is recommended that the pipeline be installed over the canal in order not to impact the integrity or operation of the canal, and to facilitate easier maintenance, if required. The 1400 mm diameter steel pipe will serve as the pipe bridge with concrete supports on either side of the canal. An air valve will have to be installed at the high point created by the canal crossing. The air valve will also serve as an access point into the pipeline for maintenance purposes.

3.6 Syphon under Sundays River

An additional syphon under the Sundays River on the existing Nooitgedagt WTW supply pipeline is recommended to:

- reduce the risk of supply failure in the event of damage to the existing syphon; and
- to mitigate the risk of the new balancing storage being located on the opposite side of the river, relative to the WTW.

The additional syphon under the Sundays River will be concrete encased. The top of the reinforced pipe encasement should be below riverbed level. The length of the encasement is assumed to be approximately 105 m (the same as the existing pipeline).

An air valve chamber and a scour valve chamber will have to be installed, and tie-ins made into the existing pipeline. The air valve will also serve as an access point into the new pipeline for maintenance purposes. The tie-ins will comprise 1400 mm x 1400 mm equal tees that will be cut into the existing pipeline and installed on the new syphon pipeline. Isolating valves will be provided so that the new syphon can be isolated, as it will only be used if the existing syphon is damaged or when maintenance is required.

3.7 Construction cost estimate

A detailed cost estimate of the required conveyance infrastructure is provided in **Appendix B**. **Table 3-1** provides a summary of the costs for the various components and types of work discussed above.

No	Description	Amount (to nearest 1000)		
1	Kirkwood Canal off-take	R	1 873 000	
2	Inlet/outlet to Coerney Dam	R	6 831 000	
3	Main connecting pipeline	R	32 760 000	
4	Tie-in to Nooitgedagt pipeline	R	1 277 000	
5	Crossing of Middle Addo canal	R	812 000	
6	Syphon under Sundays River	R	11 205 000	
	Sub total (a)	R	54 759 000	
7	Preliminary and General Items (50%)	R	27 379 000	
	Sub total (b)	R	82 138 000	
8	Contingencies (15%)	R	12 321 000	
	Total	R	94 458 000	

Table 3-1: Cost estimate summary for the conveyance infrastructure (excluding VAT)

3.8 Operation and maintenance costs

The operation and maintenance costs (**Table 3-2**) of the various conveyance infrastructure components have been included as an annual cost based on a percentage of the construction value.

The components are divided into two sections.

- Civil works, which includes the pipelines and all concrete work at tie-ins and chambers,
- The mechanical works, which includes valves and gates.



A simple annual cost estimate for the two categories has been determined, based on a percentage of the construction value. The construction value used includes 15% contingencies and excludes the 50% preliminary and general charge items.

No	Description	Percentage	Construction value (rounded to nearest 1000)		Ann (round	ual Amount ded to nearest 1000)
1	Civil works	0.5%	R	56 809 000	R	284 000
2	Mechanical works	4.0%	R	6 164 000	R	247 000
Total			R	62 972 000	R	531 000

 Table 3-2: Operation and maintenance costs for the conveyance infrastructure (excluding VAT)

4 Other miscellaneous project costs

4.1 Land acquisition

The surface area required for the dam wall and spillway is 4.4 ha. The surface area covered by the basin when at the recommended design flood (RDF) water level is 80.1 ha. This equates to a total area of 84.5 ha. Assuming that it is undeveloped irrigable land, at a cost of R 250 000 per hectare, the cost of land acquisition amounts to R 21 250 000.

The surface area for the 1860 m long pipeline, including a 15 m wide servitude, is 2.8 ha. Thus, the estimated cost of land acquisition for conveyance infrastructure, at a cost of R 250 000 per hectare, amounts to R 700 000.

4.2 **Professional fees**

An allowance of 10% of the construction cost has been included to account for professional fees for engineering services, site supervision, dam safety requirements, environmental compliance and other specialist requirements.

4.3 Cost estimate

Other miscellaneous project costs are summarised in Table 4-1.

No	Description	Amount	
1	Land acquisition	R	21 950 000
2	Professional fees for dam (10% of construction cost)	R	29 748 000
3	Professional fees for conveyance infrastructure (10% of construction cost)	R	9 446 000
	Total	R	61 144 000

Table 4-1: Other miscellaneous project costs (excluding VAT)

5 Project cost estimate

The cost estimate for the construction of the balancing dam, the conveyance infrastructure, other miscellaneous costs, professional fees and land acquisition costs have been discussed in the preceding sections. **Table 5-1** shows a summary of the total cost estimate.

No	Description	Amount	
1	Balancing dam	R	297 481 000
2	Conveyance infrastructure	R	94 456 000
3	Professional fees	R	39 194 000
	Value Added Tax (15%)	R	64 670 000
4	Land acquisition	R	21 950 000
	TOTAL (January 2020 prices)	R	517 753 000

Table 5-1: Project cost estimate

The project cost estimate for commencement in 2025 with escalation of 6.5% from the base year of 2020 is shown in **Table 5-2**.

Year	Escalation rate	Notes	Present cost	Future cost (Base year 2020)	
2025	6.5%	Land acquisition + 40% Professional fees	R 39 979 240	R 54 775 000	
2026	6.5%	33% Construction value + 20% Professional fees	R 159 257 903	R 232 380 000	
2027	6.5%	33% Construction value + 20% Professional fees	R 159 257 903	R 247 485 000	
2028	6.5%	33% Construction value + 20% Professional fees	R 159 257 903	R 263 571 000	
		TOTAL	R 517 753 000	R 798 211 000	

Table 5-2: Project cost estimate with escalation

References

Department of Water and Sanitation, South Africa 2017, Environmental Constraints Analysis. Prepared by Aurecon South Africa (Pty) Ltd as part of the Support of the Water Reconciliation Strategy for the Algoa Water Supply System.

Department of Water and Sanitation, South Africa 2017, Identification of Options for Balancing Storage. Prepared by Aurecon South Africa (Pty) Ltd with AfriCoast Consulting Engineer's assistance as part of the Support of the Water Reconciliation Strategy for the Algoa Water Supply System.

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Department of Water and Sanitation, South Africa 2018. Coerney Dam Contour Survey (EC 003/2018). Survey Services: Southern Operations (National Water Resource Infrastructure).

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Appendix A: Cost estimate for the balancing dam



EARTHFILL DAM COST MODEL Spillway on left abutment 20 m Max wall height (m)= NOCL= RL 102.0 m 27-Mar-20 FSL= Proposed Lower Coerney : 4.7 Mm3 gross storage capacity RL 98.2 m Lower Coerney Dam Costing v 18 - Spillway 50 m - Left abutment V5.xlsx Crest width (m)= 5 m DESCRIPTION Clearing (d) 1 (a) sparse ha 25 000 0.0 0 (b) bush 35 000 2.2 76 000 ha 2.2 119 000 (c) trees ha 55 000 2 River diversion Sum 10 000 000 10 000 000 3 Excavation (d) (a) Bulk (i) all materials m³ 80 136 674 10 934 000 (ii) extra over for rock m³ 400 13 667 5 467 000 (b) Confined 1 786 000 (i) all materials m³ 200 8 9 2 8 (ii) extra over for rock 536 000 m³ 600 893 (c) Preparation of solum (i) all materials m² 35 36 201 1 267 000 (ii) extra over for rock 150 1 810 272 000 m² Drilling & Grouting (d) Δ 1 500 563 000 (a) Curtain grouting m drill 376 (b) Consolidation grouting m drill 1 500 114 000 76 (c) Slurry trench - fill m3 0 0 5 Embankment (d) (a) Earthfill т³ 110 238 173 26 199 000 (b) Filters m³ 700 5 0 9 6 3 567 000 (c) Rip rap т³ 800 10 084 8 067 000 (d) Overhaul beyond 1km (one way) m³km 10 C 0 т³ 600 4 7 4 5 2 847 000 (e) Toe drain 1 037 000 (f) Spillway channel protection with reno m³ 3 000 346 Concrete Works (c) 6 (a) Formwork (i) gang formed m² 675 6 6 6 1 4 496 000 10 905 9 270 000 (ii) intricate m² 850 (b) Concrete (i) mass m³ 2 700 8 5 3 8 23 052 000 (ii) structural m³ 3 000 900 2 700 000 (c) Reinforcing 16 500 791 13 052 000 t Mechanical Items (a) Valves & gates (m) 2 000 000 10 000 000 No 5 (b) Cranes & hoists (m) Sum 5 000 000 5 000 000 (c) Structural steelwork (c) 2 000 000 2 000 000 Sum (d) Outlet pipe (SS316) (c) 15 000 368 5 525 000 m SUB-TOTAL 147 946 000



Proposed Lower Coerney : 4.7 Mm3 gross storage capacity

No	DESCRIPTION	UNIT	RATE Jan 20 Rand	QUANTITY	AMOUNT Rounded to nearest 1000 Rand, Excl VAT	
8	Miscellaneous (% of 1-9) (c)	%	6		8 877 000	
	SUB TOTAL A				156 823 000	
9	Preliminary & General (% of sub-total A)	%	50		78 412 000	
10	Preliminary works					
	(a) Access road (construction and maintenance, 3yr) (c)	km	1 500 000	1.50	2 250 000	
	(b) Electrical supply to site (c)	Sum	500 000		500 000	
	SUB TOTAL B				237 985 000	
11	Contingencies (% of sub total B)	%	25		59 496 000	
	SUB TOTAL C (CONSTRUCTION COST)				R 297 481 000	
12	Professional fees (% of sub total C)	%	10		29 748 000	
	SUB TOTAL D				327 229 000	
	VAT 15%				49 084 000	
	TOTAL CONSTRUCTION PHASE COST				R 376 313 000	
13	Cost of relocations	Sum	0		0	
14	Cost of land acquisition					
	(a) Irrigated (b) Dryland farming (c) Undeveloped (d) Homesteads	ha ha ha No	1 000 000 600 000 250 000 0	0.0 0.0 85.0	0 0 21 250 000 0	
	TOTAL PROJECT COST (as at Jan 2020)				R 397 563 000	
	(Rounded to nearest R 100 000)				R 397 600 000	
NOTE	NOTE					
	(c) included in Civil maintenance cost (m) included in Mechanical maintenance cost					
	(d) included in Dam maintenance cost					

-2-

Appendix B: Cost estimate for the conveyance infrastructure

CONVEYANCE INFRASTRUCTURE COST

25-Mar-20

No	DESCRIPTION	UNIT	RATE Jan 20 Rand	QUANTITY	AMOUNT (Excl VAT) Rand
1	Steel pipelines		10,000	10	400.000
	a) DN 1000 b) DN 1400	m m	12 000 19 200	10 1700	120 000 32 640 000
2	Tie-in to Nooitgedagt				
	a) DN1400 Butterfly valve*	No	500 000	1	500 000
	b) DN1400 Equal tee	No	250 000	1	250 000
	c) DN1400 Restrained Flange Adaptor	No	50 000	1	50 000
	d) DN1400 Puddle Pipe Plain Ended, 1760	No	80 000	1	80 000
	e) DN1400 Puddle Pipe Flanged one end, 2260	NO	90 000	1	90 000
	r) Chamber (20m2)	Sum	150 000		150 000
	g) Champer (28m3)	Sum	156 800		156 800
3	Coerney Dam Inlet/Outlet Chamber				
Ũ	a) DN 1200 90 Bend	No	115 000	1	115 000
	b) DN 1200 22.5 Bend	No	80 500	2	161 000
	c) DN 1200 NRV - PN 16	No	2 400 000	1	2 400 000
	d) DN 1200 Butterfly valve*	No	427 000	3	1 281 000
	e) Flanges	No	80 000	10	800 000
	f) DN 150 AV*	No	13 600	2	27 200
	g) DN 150 RSV*	No	4 650	2	9 300
	h) DN 1400 x DN1200 Reducer	No	150 000	1	150 000
	i) DN 1200 x DN1000 Reducer	No	130 000	2	260 000
	j) DN 1200 Restrained Flange Adaptor	No	40 000	2	80 000
	k) DN 1200 Flanged Equal tee	No	187 500	3	562 500
	I) DN 1200 Puddle pipe, 2685	No	60 000	1	60 000
	m) DN 1200 Puddle pipe, 4570	No	75 000	1	75 000
	n) DN 1200 Puddle pipe, 1800	NO	55 000	1	55 000
	o) DN 1000 Puddle pipe, 2365	NO No	50 000	1	50 000
	(p) DN 1000 Fudule pipe, 665 a) Chamber (50m3)	Sum	40 000	1	45 000
		oum	700 000		100 000
4	Middle Addo Canal Crossing				
	a) DN 1400 45° Bends	No	103 500	4	414 000
	b) Concrete plinths	No	56 000	5	280 000
	c) DN 150 AV*	No	13 600	1	13 600
	a) DN 150 RSV*	N0 Sum	4 650	1	4 650
		Sum	100 000		100 000
5	Syphon Sundays River				
	a) DN1400	m	19 200	150	2 880 000
	b) Concrete Encasement	m³	2 800	600	1 680 000
	c) Chambers	No	500 000	2	1 000 000
	d) DN 150 AV*	No	13 600	1	13 600
	e) DN 150 RSV*	No	4 650	1	4 650
	f) DN 200 SV*	No	15 000	1	15 000
	g) DN 1400 x DN 200 Tee	No	60 000	1	60 000
	h) Spare DN 1400 Pipes	m	19 200	60	1 152 000
	I) Allowance for river diversion / coffer dam	Sum	1 500 000		1 500 000
	j) DN 1400 isolating valves*	No	500 000	4	2 000 000
	K) UN 1400 X UN 1400 equal tee	INO No	250 000	2	500 000
	n) out into existing pipe and repair		150 000	2	300 000
	מטוושם (ווו	Sulli	100 000		100 000

No	DESCRIPTION	UNIT	RATE Jan 20 Rand	QUANTITY	AMOUNT (Excl VAT) Rand
6	Kirkwood Canal				
	a) Weir Chamber (20m3)	Sum	224 000		224 000
	b) Flowmeter Chamber (41m3)	Sum	459 200		459 200
	c) DN 1200 flowmeter*	No	300 000	1	300 000
	d) DN 1200 restrained flange adaptor	No	40 000	1	40 000
	e) Sluice Gate*	No	400 000	1	400 000
	f) DN 1400 x DN 1200 Reducer	No	150 000	1	150 000
	g) DN 1200 Puddle pipe, flanged one end, 3600	No	100 000	1	100 000
	h) Break and repair existing canal	Sum	80 000		80 000
	i) DN 1200 Puddle pipe, flanged one end, 6000	No	120 000	1	120 000
	SUB-TOTAL A				54 758 500
					011000000
	Preliminary & General				
	(% of sub-total A)	%	50		27 379 250
	SUB TOTAL B				82 137 750
	Contingencies	%	15		12 320 663
	(% of sub total B)				
	SUB TOTAL C				94 458 413
	Professional fees	%	10		9 445 841
	(% of sub total C)	,,,	10		0 440 041
	SUB TOTAL D				103 904 254
	VAT 15%				15 585 638
	TOTAL CONSTRUCTION COST				R 119 489 892
	(Pounded to pearest P 100 000)				P 119 500 000
	(Rounded to hearest R 100 000)				R 119 500 000

* Items included in mechanical works for operation and maintenance

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